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European Technical Assessment

ETA-22/0772
of 03.04.2023

General part

Technical Assessment Body issuing the European Technical Assessment

Österreichisches Institut für Bautechnik (OIB)
Austrian Institute of Construction Engineering

Trade name of the construction product

Hilti screws S-WCF, S-WXF, S-WCP, S-WWP
and S-WDF

Product family to which the construction product belongs

Screws for use in timber constructions

Manufacturer

Hilti AG
Feldkircherstrasse 100
FL 9494 Schaan
Principality of Liechtenstein

Manufacturing plant

Hilti AG, Plant 0320

This European Technical Assessment contains

51 pages including 9 Annexes, which form an integral part of this assessment.

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

European Assessment Document (EAD)
130118-01-0603 "Screws and threaded rods for use in timber constructions".

Remarks

Translations of the European Technical Assessment in other languages shall fully correspond to the original issued document and should be identified as such.

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Specific parts

1 Technical description of the product

This European Technical Assessment (ETA) applies to the screws for use in timber constructions “Hilti screws S-WCF, S-WXF, S-WCP, S-WWP and S-WDF” hereinafter referred to as Hilti screws. Hilti screws are self-tapping screws divided into a drill tip, optionally a compressor and/or cutting groove, thread, optionally a friction part, shank, and head of the screw. The screws are made from special carbon steel and hardened. They are anti-friction coated and are electrogalvanized and passivated (yellow or blue), provided with a zinc-nickel coating or hot-dip galvanized. The washers are made from carbon steel. Possible outer thread diameters as well as overall lengths for the Hilti screws are given in Table 1.

A bending angle of 45° is reached for all screws.

The screws and washers correspond to the specifications given from Annex 1 to Annex 5. The material characteristics, dimensions and tolerances of the product not indicated in these Annexes, are given in the technical file¹ of the European Technical Assessment.

Table 1: Possible outer thread diameter and overall length of screws

Type of Hilti screws	Outer thread diameter		Overall length	
	min.	max.	min.	max.
	mm	mm	mm	mm
S-WCF-H-8xL Z S-WCF-H-10xL Z S-WCF-H-12xL Z	8	12	50 50 60	1 000
S-WXF-H/S-8xL Z * S-WXF-H-10xL Z S-WXF-H-12xL Z	8	12	50 50 60	1 000
S-WCP-S-4xL Z S-WCP-S-5xL Z S-WCP-S-6xL Z S-WCP-S-8xL Z S-WCP-S-10xL Z S-WCP-S-12xL Z	4	12	30 30 50 50 60 80	600
S-WWP-S-6xL Z S-WWP-S-8xL Z S-WWP-S-10xL Z S-WWP-S-12xL Z	6	12	50 50 60 80	600
S-WDF-S-12xL Z	12	12	60	1 000

* H/S ... with / without half tip

¹ The technical file of the European Technical Assessment is deposited at Österreichisches Institut für Bautechnik.

2 Specification of the intended use(s) in accordance with the applicable European Assessment Document

2.1 Intended use

The screws are used for connections in load bearing timber structures between wood-based members or between those members and steel members:

- Solid timber of softwood of strength class C14 or better and solid timber of hardwood of strength class D18 or better according to EN 338² and EN 14081-1,
- Glued laminated timber and glued solid timber of softwood of strength class GL20 or better according to EN 14080 or glued laminated timber of hardwood according to European Technical Assessments (ETA) or national provisions that apply on the installation site,
- Laminated veneer lumber LVL according to EN 14374,
- Cross laminated timber according to European Technical Assessments or national provisions that apply on the installation site.
- Cement-bonded particle boards according EN 634-1 and EN 13986 or ETA or national provisions that apply on the installation site.

The screws may be used for connecting the following wood-based panels to the timber members mentioned above:

- Laminated veneer lumber LVL according to EN 14374 or ETA,
- Solid wood panels according to EN 13353 and EN 13986 or ETA,
- Plywood according to EN 636 and EN 13986 or ETA,
- Oriented strand boards, OSB, according to EN 300 and EN 13986 or ETA,
- Particleboards according to EN 312 and EN 13986 or ETA,
- Fibreboards according to EN 622-2, EN 622-3 and EN 13986 or ETA,
- Cement-bonded particle boards according EN 634-1 and EN 13986 or ETA or national provisions that apply on the installation site,
- Engineered wood products according to ETA, provided that the ETA for the product provides provisions for the use of self-tapping screws and these provisions are applied.

Compression and tension reinforcement perpendicular to the grain with fully threaded screws as well as shear reinforcement with fully threaded screws with a diameter $d \geq 8$ mm is allowed.

In addition, screws with $6 \text{ mm} \leq d \leq 12 \text{ mm}$ may be used for fixing of thermal insulation on rafters and walls.

For engineered wood products according to a European Technical Assessment (ETA) including provisions for the use of self-tapping screws, the provisions of the ETA of the engineered wood product apply.

The product shall be subjected to static and quasi static actions only.

The product is intended to be used in service classes 1, 2 and 3 according to EN 1995-1-1. The scope of the screws regarding resistance to corrosion shall be defined according to national provisions that apply at the installation site considering environmental conditions.

Hot-dip galvanised screws with a minimum thickness of the zinc coating of 55 μm may be used in conditions defined by service class 3.

² Reference documents are listed in Annex 9.

2.2 General assumptions

The screws for use in timber constructions are manufactured in accordance with the provisions of the European Technical Assessment using the manufacturing process as identified in the inspection of the manufacturing plant by Österreichisches Institut für Bautechnik and laid down in the technical file.

The manufacturer shall ensure that the requirements in accordance with the Clauses 1, 2 and 3 as well as with the Annexes of the European Technical Assessment are made known to those who are concerned with design and execution of the works.

Design

The European Technical Assessment only applies to the manufacture and use of the screws for use in timber constructions. Verification of stability of the works including application of loads on the products is not subject to the European Technical Assessment.

The following conditions shall be observed:

- Design of Hilti screws is carried under the responsibility of an engineer experienced in such products.
- Design of the works shall account for the protection of Hilti screws to maintain service classes 1, 2 and 3 according to EN 1995-1-1 or national provisions that apply on the installation site.
- Hilti screws are installed correctly.

Design of the screws for use in timber constructions may be according to EN 1995-1-1, taking into account of Annex 5 to Annex 8 of the European Technical Assessment. Hereby, the outer thread diameter d is used as nominal diameter d or rather effective diameter d_{ef} and l_{ef} is the threaded part in the timber member including point.

Standards and regulations in force at the place of use shall be considered.

Packaging, transport, storage, maintenance, replacement and repair

Concerning product packaging, transport, storage, maintenance, replacement and repair it is the responsibility of the manufacturer to undertake the appropriate measures and to advise his clients on the transport, storage, maintenance, replacement and repair of the product as he considers necessary.

Installation

It is assumed that the product will be installed according to the manufacturer's instructions or (in absence of such instructions) according to the usual practice of the building professionals.

The screws are either driven into the wood-based member of softwood without pre-drilling or in predrilled holes with a diameter not exceeding the inner thread diameter or rather into the wood-based member of hardwood in predrilled holes with a diameter minimally exceeding the inner thread diameter.

The screw holes in steel members shall be pre-drilled with an adequate diameter greater than the outer thread diameter.

The minimum penetration length of screws in the load-bearing wood-based members shall be $4d$.

Screws made of carbon steel with an outer thread diameter $5\text{ mm} \leq d \leq 12\text{ mm}$ may be driven into laminated veneer lumber LVL of beech or related products of hardwood with predrilling.

At least four screws shall be used in a connection with screws ($4\text{ mm} \leq d \leq 12\text{ mm}$) inserted in the timber member with an angle between screw axis and grain direction of less than 15° . The penetration length of the threaded part of the partly or fully threaded screw shall be at least $20d$.

The use of only one screw in load-bearing connections is possible for screws ($4\text{ mm} \leq d \leq 12\text{ mm}$) loaded in axial direction and angles between grain direction and screw axis $\alpha \geq 15^\circ$ provided that a

minimum penetration length of the threaded part of the screw of 20 d can be ensured. Hereby, the load-bearing capacity of the screw must be reduced by 50%. This reduction is not necessary for screws used as reinforcement perpendicular to the grain of wood-based members.

To ensure a proper installation for screws with lengths of more than 800 mm a guiding hole of 5 d is recommended.

For mounting of steel plates and wood-based panels the screw head must be placed on top of these members.

The structural members which are connected with Hilti screws shall

- be in accordance with Clause 2.1;
- ensure minimum spacing and edge distances in accordance with EN 1995-1-1 and Annex 5.

2.3 Assumed working life

The provisions made in the European Technical Assessment (ETA) are based on an assumed intended working life of Hilti screws of 50 years, when installed in the works, provided that the screws are subject to appropriate installation, use and maintenance (see Clause 2.2). These provisions are based upon the current state of the art and the available knowledge and experience³.

The indications given as to the working life of the construction product cannot be interpreted as a guarantee neither given by the product manufacturer or his representative nor by EOTA nor by the Technical Assessment Body, but are regarded only as a means for choosing the appropriate products in relation to the expected economically reasonable working life of the works.

³ The real working life of a product incorporated in a specific works depends on the environmental conditions to which that works is subject, as well as on the particular conditions of the design, execution, use and maintenance of that works. Therefore, it cannot be excluded that in certain cases the real working life of the product can also be shorter than the assumed working life.

3 Performance of the product and reference to the methods used for its assessment

3.1 Essential characteristics of the product

Table 2: Essential characteristics of the product and product performance

No	Essential characteristic	Product performance
Basic requirement for construction works 1: Mechanical resistance and stability ¹⁾		
1	Dimensions	Annex 1 to Annex 4
2	Characteristic yield moment	Annex 5
3	Bending angle	Clause 1
4	Characteristic withdrawal parameter	Annex 5
5	Characteristic head pull-trough parameter	Annex 5
6	Characteristic tensile strength	Annex 5
7	Characteristic yield strength	Annex 5
8	Characteristic torsional strength	Annex 5
9	Insertion moment	Annex 5
10	Spacing, end and edge distances of the screws and minimum thickness of the wood based material	Annex 5
11	Slip modulus for mainly axially loaded screws	Annex 5
12	Durability against corrosion	3.1.1
Basic requirement for construction works 2: Safety in case of fire		
13	Reaction to fire	3.1.2
Basic requirement for construction works 4: Safety and accessibility in use		
14	Same as BWR 1	
¹⁾ These characteristics also relate to basic requirement 4 for construction works.		

3.1.1 Durability against corrosion

The product is intended to be used in service classes 1, 2 and 3 according to EN 1995-1-1.

The screws and washers made from carbon steel are electrogalvanized and yellow or blue passivated, coated with a zinc-nickel coating or hot-dip galvanized. The minimum thickness of the zinc coating of the screws is 5 µm and the minimum thickness of the zinc-nickel coating is 4 µm. The minimum thickness of the zinc coating of hot-dip galvanized screws is 55 µm.

Durability of Hilti screws is in accordance with EN 1995-1-1 or national provisions that apply on the installation site.

3.1.2 Reaction to fire

Hilti screws are made from steel classified as Euroclass A1 in accordance with Commission Decision 96/603/EC, as amended by Commission Decision 2000/605/EC.

3.2 Assessment methods

3.2.1 General

The assessment of the essential characteristics in Clause 3.1 of the screws for use in timber constructions for the intended use, and in relation to the requirements for mechanical resistance and stability, for safety in case of fire and for safety and accessibility in use in the sense of the basic requirements for construction works № 1, 2 and 4 of Regulation (EU) № 305/2011 has been made in accordance with the European Assessment Document EAD 130118-01-0603, "Screws and threaded rods for use in timber constructions".

3.2.2 Identification

The European Technical Assessment for the screws for use in timber constructions is issued on the basis of agreed data that identify the assessed product. Changes to materials, to composition, to characteristics of the product, or to the production process could result in these deposited data being incorrect. Österreichisches Institut für Bautechnik should be notified before the changes are implemented, as an amendment of the European Technical Assessment is possibly necessary.

4 Assessment and verification of constancy of performance (thereinafter AVCP) system applied, with reference to its legal base

4.1 System of assessment and verification of constancy of performance

According to Commission Decision 97/176/EC the system of assessment and verification of constancy of performance to be applied to "Hilti screws S-WCF, S-WXF, S-WCP, S-WWP and S-WDF" is System 3. System 3 is detailed in Commission Delegated Regulation (EU) № 568/2014 of 18 February 2014, Annex, 1.4., and provides for the following items

- (a) The manufacturer shall carry out factory production control.
- (b) The notified laboratory shall assess the performance on the basis of testing (based on sampling carried out by the manufacturer), calculation, tabulated values or descriptive documentation of the construction product.

4.2 AVCP for construction products for which a European Technical Assessment has been issued

Notified bodies undertaking tasks under System 3 shall consider the European Technical Assessment issued for the construction product in question as the assessment of the performance of that product. Notified bodies shall therefore not undertake the tasks referred to in point 4.1 (b).

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

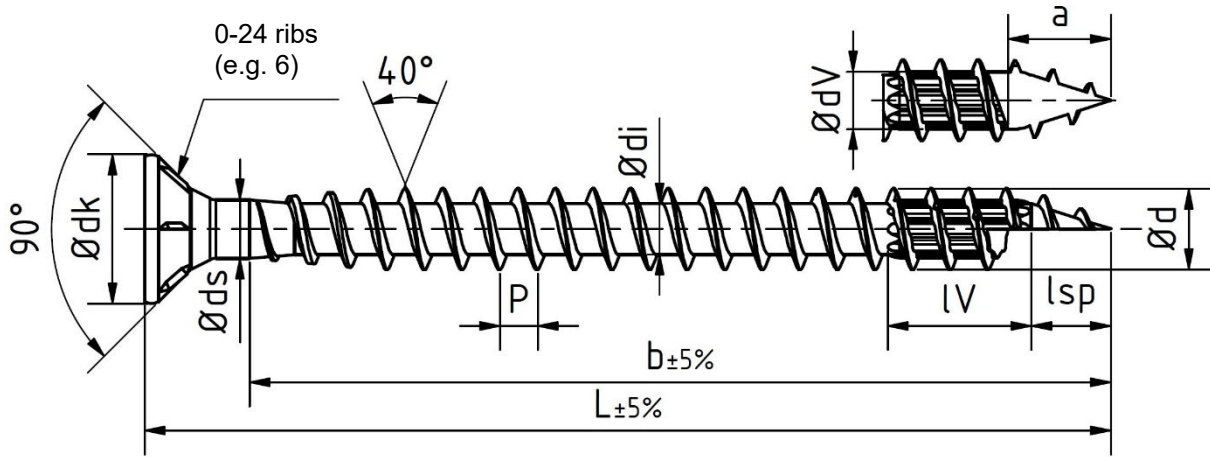
5.1 Tasks for the manufacturer

5.1.1 Factory production control

In the manufacturing plant the manufacturer shall establish and continuously maintain a factory production control. All procedures and specifications adopted by the manufacturer shall be documented in a systematic manner. The factory production control shall ensure the constancy of performances of Hilti screws with regard to the essential characteristics.

The manufacturer shall only use raw materials supplied with the relevant inspection documents as laid down in the control plan. The incoming raw materials shall be subject to controls by the manufacturer before acceptance. Check of incoming materials shall include control of inspection documents presented by the manufacturer of the raw materials.

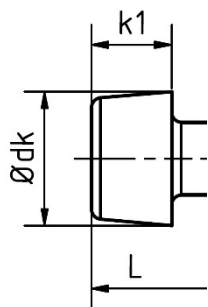
Full threaded screws, countersunk head, half tip: S-WCF-H (alternative with full tip: S-WCF-S)
 (all dimensions in mm)



Dim	Ødk	Øds ±5%	Ød	Ødi ±5%	P ±10%	lsp	a	ØdV ±10%	L	b
8,0	15,0 ±1,2	5,9	8,0 ±0,4	5,1	3,8	8,2 ±2,1	11,0 ±2,5	6,0	50-400	L-10
									>400	L-23
10,0	18,5 ±1,5	7,1	10,0 ±0,6	6,3	4,6	10,1 ±2,3	13,0 ±3,0	7,1	50-300	L-12
									>300	L-24
12,0	21,0 ±2,0	8,2	12,0 ±0,7	7,0	6,0	11,2 ±2,6	15,0 ±3,0	7,9	60-300	L-20
									>300	L-25

Compressor IV = 2P to 4P, number of flanks: 4-8

Full threaded screws with cylinder head: S-WXF-S (with full tip), S -WXF-H (with half tip)



Dim	Ødk	k1
8,0	10,2 ±0,51	7,5 ±1,0
10,0	13,4 ±0,67	8,0 ±1,0
12,0	14,2 ±0,71	10,0 ±1,5

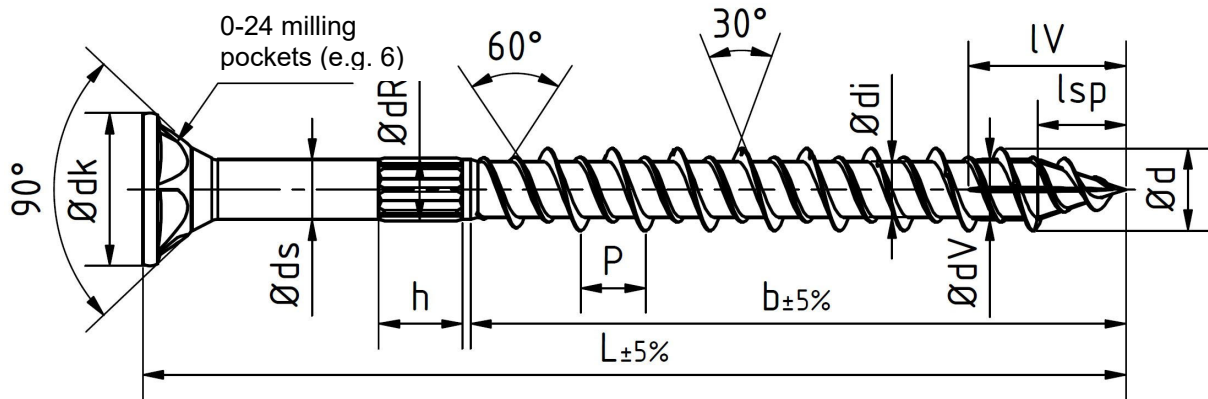
Hilti screws

S-WCF-S, S-WCF-H
 S-WXF-S, S-WXF-H
 S-WDF-S, S-WDF-H

Annex 1

of European Technical Assessment
 ETA-22/0772 of 03.04.2023

Partly threaded screws, countersunk head: S-WCP-S (with full tip)
 (all dimensions in mm)



Dim	Ødk	Øds ±5%	Ød	Ødi	P ±10%	ØdR ±10%	h	lsp	ØdV ±10%
4,0	8,0 ±0,7	2,8	4,0 ±0,20	2,45 ±0,15	3,4	3,1	6,2 ±1,0	4,6 ±1,5	2,9
5,0	10,0 ±0,8	3,6	5,0 ±0,25	3,25 ±0,16	4,2	3,9	8,2 ±1,0	6,0 ±1,7	3,7
6,0	12,0 ±0,9	4,3	6,0 ±0,30	4,00 ±0,20	5,0	4,7	10,2 ±1,0	7,3 ±1,9	4,4
8,0	15,0 ±1,2	5,9	8,0 ±0,40	5,35 ±0,28	6,7	6,2	10,2 ±1,0	8,2 ±2,1	6,0
10,0	18,5 ±1,5	7,1	10,0 ±0,60	6,80 ±0,34	7,9	7,7	10,2 ±1,0	10,1 ±2,3	7,1

Compressor IV = 2P to 4P, number of flanks: 4-8

Screw length L and thread length b									
Dim. 4,0		Dim. 5,0		Dim. 6,0		Dim. 8,0		Dim. 10,0	
L	b	L	b	L	b	L	b	L	b
30-35	20	30-35	20	50	30	50	30	60-70	40
40-45	25	40	25	60-70	40	60-70	40	80	50
50	30	50	30	80-90	50	80-90	50	100	60
60-70	35	60-70	40	100-110	60	100	60	120-160	80
		80-90	50	120-300	70	120-160	80	180-600	100
		100-120	60			180-600	100		

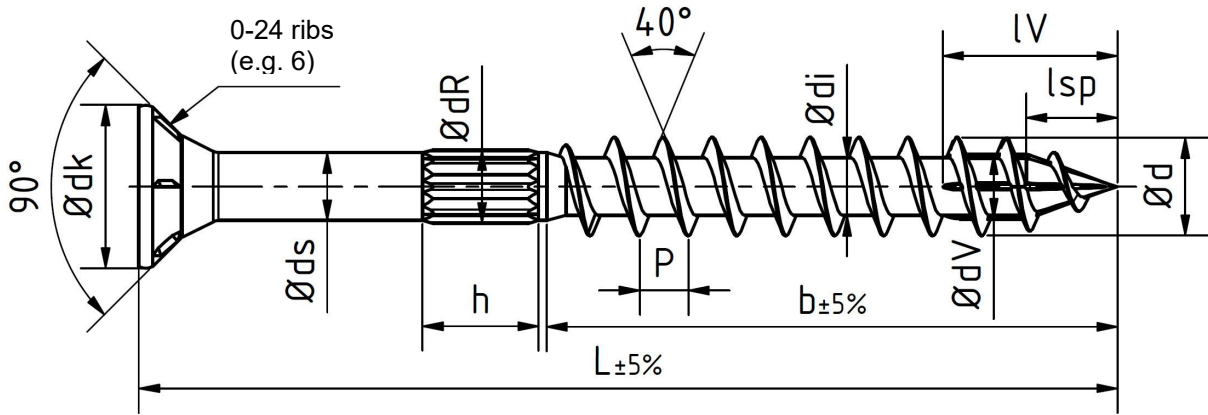
Hilti screws

S-WCP-S
 S-WWP-S

Annex 2

of European Technical Assessment
 ETA-22/0772 of 03.04.2023

Partly threaded screws, countersunk head: S-WCP-S with $d = 12\text{mm}$ (with full tip)
 (all dimensions in mm)

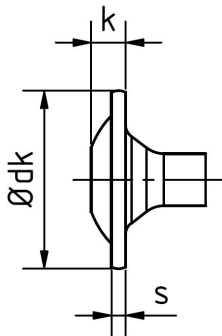


Dim	Ødk	Øds ±5%	Ød	Ødi ±5%	P ±10%	ØdR ±10%	h	lsp	ØdV ±10%
12,0	21,0 ±2,0	8,2	12,0 ±0,7	7,0	6,0	9,0	14,2 ±1,0	11,2 ±2,6	7,9

Compressor IV = 2P to 4P, number of flanks: 4-8

Screw length L and thread length b	
Dim. 12.0	
L	b
80	50
100	60
120-160	80
180-280	100
300-600	120

Partly threaded screws with washer head: S-WWP-S (with full tip)



Dim	Ødk	k	s
12,0	27,0 ±2,0	4,7 ±1,2	2,5 ±0,9

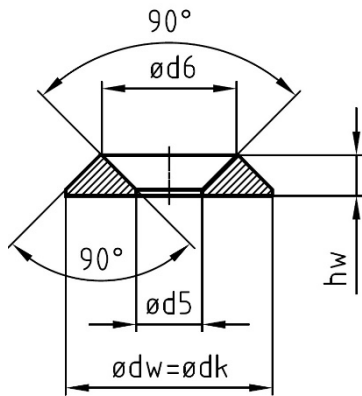
Hilti screws

S-WCP-S with $d = 12\text{ mm}$
 S-WWP-S with $d = 12\text{ mm}$

Annex 3

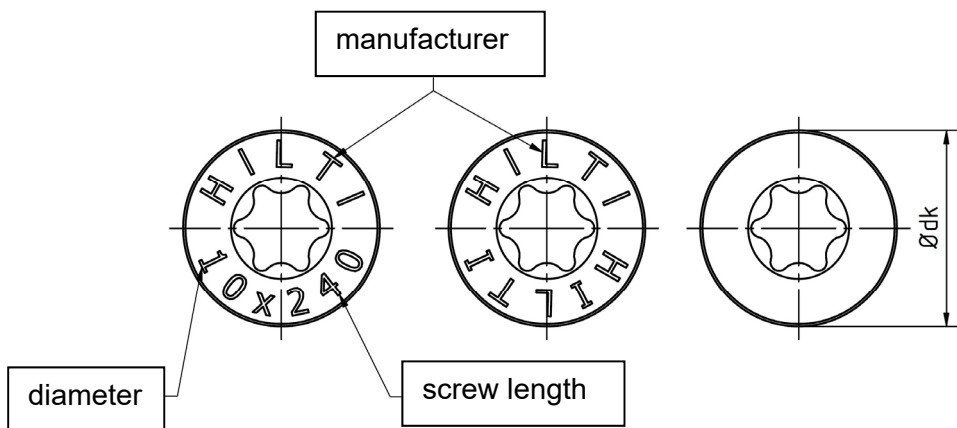
of European Technical Assessment
 ETA-22/0772 of 03.04.2023

Washer (for screws with 90° head, only)



Dim	$\varnothing dw = \varnothing dk$	$\varnothing d5$	$\varnothing d6$	hw
6,0	22,0 ±2.0	8,5 ±2.0	13,5 ±1.5	4,5 ±1.0
8,0	28,0 ±2.0	10,0 ±2.0	17,5 ±2.0	5,5 ±1.0
10,0	35,0 ±3.0	12,0 ±2.0	22,5 ±2.2	6,5 ±1.5
12,0	42,0 ±3.0	14,0 ±2.0	25,0 ±2.5	7,5 ±1.5

Head mark (example)



T-Drive size

Dim	T
4,0	T10 / T15 / T20
5,0	T20 / T25 / T30
6,0	T20 / T25 / T30
8,0	T30 / T40
10,0	T40 / T50
12,0	T40 / T50 / T55

Hilti screws

Drive types, washer and head mark

Annex 4

of European Technical Assessment
ETA-22/0772 of 03.04.2023

A.5 Load bearing capacity of Hilti screws

The characteristic load bearing capacities in Tables A5.1 to A5.2 are given for timber of strength class C24 according to EN 338 ($\rho_{k,ref} = 350 \text{ kg/m}^3$) unless specified otherwise in the following.

If relevant, the design block shear failure resistance of a timber member has to be proven.

Table A5.1: Characteristic load bearing capacities of Hilti screws S-WCF, S-WXF and S-WDF; screw diameter 8 to 12 mm

Product characteristic			Screw diameter		
			8	10	12
Max. length	l_{max}	mm	1000	1000	1000
Characteristic tensile strength	$f_{tens,k}$	kN	24.1	40.0	46.7 45.0 ¹⁾
Characteristic yield moment	$M_{y,k}$	Nm	20.3	36.7	48.5
Characteristic withdrawal parameter angle screw-axis to grain: 90° ($\rho_{k,ref} = 350 \text{ kg/m}^3$)	$f_{ax,k,90^\circ}$	N/mm ²	13.1	12.5	11.2
Characteristic yield strength	$f_{y,k}$	N/mm ²	950 (carbon steel)		
Characteristic torsional strength	$f_{tor,k}$	Nm	25.8	55.0	73.0
Ratio characteristic torsional strength to mean insertion moment $\rho_{k,ref} = 450 \text{ kg/m}^3$	$f_{tor,k} / R_{tor,m}$	-	≥ 1.5	≥ 1.5	≥ 1.5
Slip modulus	K_{ser}	N/mm	see A.5.1.7		

¹⁾ S-WDF-S-12xL Z

Hilti screws

Characteristic data of the screws

Annex 5

of European Technical Assessment
ETA-22/0772 of 03.04.2023

**Table A5.2: Characteristic load bearing capacities of Hilti screws S-WCP and S-WWP;
screw diameter 4 to 12 mm**

Product characteristic			Screw diameter					
			4	5	6	8	10	12
Max. length	l_{max}	mm	70	120	300	600 ¹⁾	600 ¹⁾	600 ¹⁾
Characteristic tensile strength	$f_{tens,k}$	kN	5.0	8.8	13.1	23.3	35.0	42.0
Characteristic yield moment	$M_{y,k}$	Nm	3.1	5.9	10.7	22.6	33.6	46.9
Characteristic withdrawal parameter angle screw-axis to grain: 90° ($\rho_{k,ref} = 350 \text{ kg/m}^3$)	$f_{ax,k,90^\circ}$	N/mm ²	14.3	13.6	13.0	10.9	11.0	11.2
Characteristic yield strength	$f_{y,k}$	N/mm ²	900 (carbon steel)					
Characteristic torsional strength	$f_{tor,k}$	Nm	3.5	6.6	10.9	28.0	52.5	59.6
Ratio characteristic torsional strength to mean insertion moment $\rho_{k,ref} = 450 \text{ kg/m}^3$	$f_{tor,k} / R_{tor,m}$	-	≥ 1.5					

¹⁾ For screws with $l > 500 \text{ mm}$ a friction part is mandatory.

Hilti screws

Characteristic data of the screws

Annex 5

of European Technical Assessment
ETA-22/0772 of 03.04.2023

Table A5.3: Minimum spacing, end and edge distances of only axially loaded Hilti screws (except CLT)

Designation		Variant 1	Variant 2
Boundary condition	$a_1 \cdot a_2$	$\geq 25 d^2$	$\geq 21 d^2$
Spacing in a plane parallel to the grain	a_1	$5 d$	$7 d$
Spacing perpendicular to a plane parallel to the grain	a_2	$2.5 d$	$3 d$
Spacing between the crossing screws for a crossed screw couple perpendicular to a plane parallel to the grain	a_{cross}	$1.5 d$	
End distance of the centre of gravity of the threaded part in the timber member	$a_{1,c}$	$5 d$	
Edge distance of the centre of gravity of the threaded part in the timber member	$a_{2,c}$	$4 d$	

Provided that a minimum thickness of the cross laminated timber of $10 d$ as well as a minimum penetration length of the screws of $4 d$ in the wide face or $10 d$ in the narrow face are met, the minimum spacings, end and edge distances given in Table A5.4 apply.

Table A5.4: Minimum spacings, end and edge distances of Hilti screws in cross laminated timber (axially and/or laterally loaded)

	a_1	$a_{3,t}$	$a_{3,c}$	a_2	$a_{4,t}$	$a_{4,c}$
Wide face (see Figure A5.1)	$4 d$	$6 d$	$6 d$	$2.5 d$	$6 d$	$2.5 d$
Narrow face (see Figure A5.1)	$10 d$	$12 d$	$7 d$	$3 d$	$5 d$	$3 d$

Hilti screws

Characteristic data of the screws

Annex 5

of European Technical Assessment
ETA-22/0772 of 03.04.2023

A.5.1.3 Characteristic withdrawal parameter

The characteristic withdrawal parameter for Hilti screws for angles $0^\circ \leq \alpha \leq 90^\circ$ between screw-axis and direction of wood-fibre may be calculated as

$$f_{ax,calc,k} = f_{ax,k,90^\circ} \cdot k_{ax} \cdot k_{sys} \cdot \left(\frac{\rho_k}{\rho_{k,ref}} \right)^{k_p}$$

$$k_{ax} = \begin{cases} 1.0 & \text{for } 30^\circ \leq \alpha \leq 90^\circ \\ 0.3 \cdot k_{gap} + \frac{\alpha}{30^\circ} (1 - 0.3 \cdot k_{gap}) & \text{for } 0^\circ \leq \alpha \leq 30^\circ \end{cases}$$

$$k_{gap} = \begin{cases} 0.9 & \text{for narrow face in CLT} \\ 1.0 & \text{other} \end{cases}$$

$$k_{sys} = \begin{cases} 1.0 & \text{for solid timber} \\ \text{see Table A5.5} & \text{for layered timber} \end{cases}$$

$$k_p = \begin{cases} 1.10 & \text{for softwood and } 15^\circ \leq \alpha \leq 90^\circ \\ 1.25 - 0.05 d & \text{for softwood and } 0^\circ \leq \alpha \leq 15^\circ \\ 1.40 & \text{for ring porous hardwood and } 0^\circ \leq \alpha \leq 90^\circ \\ 1.70 & \text{for diffuse porous hardwood and } 0^\circ \leq \alpha \leq 90^\circ \end{cases}$$

Examples for ring porous hardwoods: chestnut, ash, oak
 Examples for diffuse porous hardwoods: poplar, birch, beech

$f_{ax,k,90^\circ}$ characteristic withdrawal parameter according to Tables A5.1 to A5.2 in N/mm²

$\rho_{k,ref}$ reference characteristic density of timber raw material in kg/m³ in which the screw is driven (350 kg/m³ (C24) for Tables A5.1 to A5.2)

ρ_k characteristic density of timber in kg/m³

α angle between screw axis and grain direction

k_{sys} system factor according to Table A5.5

n number of screwed layers

Table A5.5: System factor k_{sys} depending on the number of layers n for screw insertion in GLT or CLT

n	1	2	3	4	5	≥ 6
k_{sys}	1.00	1.06	1.10	1.12	1.13	1.15

Hilti screws	Annex 5 of European Technical Assessment ETA-22/0772 of 03.04.2023
Characteristic data of the screws	

electronic copy

The characteristic withdrawal capacity of Hilti screws in the narrow face of cross laminated timber may be alternatively determined independent of the angle between screw axis and grain direction as

$$F_{ax,Rk} = 20 \cdot d^{0,8} \cdot l_{ef}^{0,9}$$

unless otherwise specified in the technical specification of the cross laminated timber.

A.5.1.4 Characteristic head pull-through capacity for timber

The characteristic head pull-through capacities for timber with a char. density $\rho_{k,ref} = 350 \text{ kg/m}^3$ and for a timber thickness $\geq 20 \text{ mm}$ is given in Table A5.6 and A5.7.

For softwood with a deviating density the characteristic head pull-through parameter shall be corrected by the factor

$$k_{dens} = \left(\frac{\rho_k}{350} \right)^{0,8}$$

Where

ρ_k Characteristic density of timber in kg/m^3

For the characteristic withdrawal parameter the correction according to A.5.1.3 applies.

Table A5.6: Characteristic head pull-through capacities of Hilti screws in structural timber for 90° heads; head diameter 8 to 21 mm

Group 1			Head diameter d_k (90° heads) ¹⁾					
Product characteristic			8	10	12	15	18.5	21
Characteristic head pull-through parameter ($\rho_{k,ref} = 350 \text{ kg/m}^3$)	$f_{head,k}$	N/mm ²	17.1	14.6	14.6	12.4	12.2	10.3

¹⁾ Linear interpolation is possible for head diameters in between the stated values

Table A5.7: Characteristic head pull-through capacities of Hilti screws in structural timber for washers and 180° heads; (head) diameter 14 to 27 mm

Group 2			Head diameter d_k (180° heads) ¹⁾			
Product characteristic			14	20	25	27
Characteristic head pull-through parameter ($\rho_{k,ref} = 350 \text{ kg/m}^3$)	$f_{head,k}$	N/mm ²	16.7	17.6	15.2	14.5

¹⁾ Linear interpolation is possible for head diameters in between the stated values

Hilti screws	Annex 5
Characteristic data of the screws	of European Technical Assessment ETA-22/0772 of 03.04.2023

A.5.1.5 Characteristic head pull-through capacity for wood based panels

The characteristic value of the head pull-through parameter for a characteristic density of 380 kg/m³ of the timber and for the following wood based panels

- Plywood according to EN 636 and EN 13986,
- Oriented strand boards, OSB, according to EN 300 and EN 13986,
- Solid wood panels according to EN 13353 and EN 13986,
- Particleboard according to EN 312 and EN 13986,
- Fibreboards according to EN 622-2, EN 622-3 and EN 13986,
- Cement-bonded particle boards according to EN 634-1 and EN 13986

is given in Table A5.8.

Table A5.8: Characteristic value of the head pull-through parameter in dependence of the thickness of the wood-based panels t_{WBP}

t_{WBP}	≤ 12 mm	12 mm $< t_{WBP} \leq 20$ mm	> 20 mm
$f_{head,k}$	8 N/mm ² *	8 N/mm ²	10 N/mm ²
* limited to 400 N complying with the minimum thicknesses of the wood based panels of 1.2 d, with d as outer thread diameter			

For plywood with a minimum of 7 layers and a minimum thickness of 18 mm, the characteristic value of the head pull-through parameter for a characteristic density of 490 kg/m³ is ($d_k \geq 18.8$ mm)

$$f_{head,k} = 16 \text{ N/mm}^2$$

In addition the minimum thicknesses of Table A5.9 apply.

Table A5.9 Minimum thicknesses of wood based panels

Wood based panel	Minimum thickness in mm
Plywood	6
Oriented strand board, OSB	8
Solid wood panels	12
Particleboard	8
Fibreboards	6
Cement-bonded particle boards	8

Hilti screws

Characteristic data of the screws

Annex 5

of European Technical Assessment
ETA-22/0772 of 03.04.2023

A.5.1.6 Compressive loading for fully threaded screws

The design load carrying capacity for Hilti screws with a full thread for an angle $30^\circ \leq \alpha \leq 90^\circ$ between screw-axis and direction of wood-fibre for axial compressive loading is given as

$$F_{ax,Rd} = \min \left(f_{ax,calc,k} \cdot d \cdot l_{ef} \cdot \frac{k_{mod}}{\gamma_M}; \kappa_c \cdot \frac{N_{pl,k}}{\gamma_{M1}} \right)$$

with

$f_{ax,calc,k}$ char. withdrawal capacity of the threaded part of the screw according to Clause A.5.1.3 in N/mm²

d outer thread diameter of the screw in mm

l_{ef} penetration length of the threaded part of the screw in the timber member in mm

k_{mod} modification factor for duration of load and moisture content according to EN 1995-1-1

γ_M partial safety factor for connections according to EN 1995-1-1

γ_{M1} partial safety factor according to EN 1993-1-1

$$\kappa_c = \begin{cases} 1.0 & \text{for } \bar{\lambda}_k \leq 0.2 \\ \frac{1.0}{k + \sqrt{k^2 - \bar{\lambda}_k^2}} & \text{for } \bar{\lambda}_k > 0.2 \end{cases}$$

$$k = 0.5 \left[1 + 0.49 \cdot (\bar{\lambda}_k - 0.2) + \bar{\lambda}_k^2 \right]$$

The related slenderness ratio

$$\bar{\lambda}_k = \sqrt{\frac{N_{pl,k}}{N_{ki,k}}}$$

with

$N_{pl,k}$ characteristic value of the plastic normal force load bearing capacity of the net cross-section, related to the inner thread diameter d_i (or shank diameter d_s if relevant) of the screws in N

$$N_{pl,k} = \frac{\pi \cdot d_i^2}{4} \cdot f_{y,k}$$

$f_{y,k}$ characteristic yield strength of Hilti screws in N/mm² according to Table A5.1 to A5.2

$N_{ki,k}$ characteristic ideal elastic buckling load in N

$$N_{ki,k} = \sqrt{c_h \cdot E_s \cdot I_s}$$

c_h elastic foundation of the Hilti screws in the wooden member in N/mm²

Hilti screws

Annex 5

Characteristic data of the screws

of European Technical Assessment
ETA-22/0772 of 03.04.2023

$$c_h = (0.19 + 0.012 \cdot d) \cdot \rho_k \cdot \left(\frac{90 + \alpha}{180} \right)$$

E_s modulus of elasticity of Hilti screws in N/mm², $E_s = 210\,000$ N/mm²

I_s area moment of inertia of Hilti screws in mm⁴

ρ_k characteristic density of the wood-based member in kg/m³

$$I_s = \frac{\pi \cdot d_i^4}{64}$$

A.5.1.7 Slip modulus for mainly axially loaded screws

The axial slip modulus $K_{ser,ax}$ of the threaded part per cutting surface for the serviceability limit state shall be taken for screws independent of angle α to the grain as

$$K_{ser,ax} = k_{HA} \cdot d \cdot l_{ef}$$

with

d outer thread diameter of the screw in mm

l_{ef} penetration length of the threaded part of the screw in the timber member in mm

k_{HA} coefficient depending on the type of wood of the wood-based member according to Table A5.10

Table A5.10: Coefficient k_{HA} depending on the type of wood of the wood-based member

Type of wood	Reference density ρ_m in kg/m ³	Coefficient k_{HA}
softwood	420	25
chestnut	530	48
ash	660	62
poplar	485	34
birch	635	54
beech	740	78
LVL beech*	840	53
* according to EN 14374 or European Technical Assessment		

The coefficients listed in Table A5.10 apply to Hilti screws installed with or without pre-drilling, provided that the pre-drilling diameter does not exceed 75% of the outer thread diameter.

Hilti screws	Annex 5 of European Technical Assessment ETA-22/0772 of 03.04.2023
Characteristic data of the screws	

electronic copy

The characteristic embedment strength of Hilti screws in the narrow face of cross laminated timber may be determined independent of the angle between screw axis and grain direction as

$$f_{h,k} = 20 \cdot d^{-0.5}$$

unless otherwise specified in the technical specification of the cross laminated timber.

A.5.2.4 Slip modulus for screws loaded perpendicular to the screw axis

The slip modulus $K_{ser,v}$ per shear joint for the serviceability limit state shall be taken for screws independent of angle α to the grain as

$$K_{ser,v} = k_v \cdot d^{1.7} \text{ in N/mm}^2$$

with

k_v coefficient depending on the direction of load on the type of the connection and the predrilling according to Table A5.13

Table A5.13: Coefficient k_v depending on the direction of load on the type of the connection and the predrilling

Direction of load	Non-predrilled		Pre-drilled	
	Wood-wood	Metal-wood	Wood-wood	Metal-wood
Parallel to the direction of the grain $K_{ser,v,0}$	32	64	$1.6 \cdot \rho_k^{0.5}$	$3.2 \cdot \rho_k^{0.5}$
Perpendicular to the direction of the grain $K_{ser,v,90}$	16	32	$0.8 \cdot \rho_k^{0.5}$	$1.6 \cdot \rho_k^{0.5}$

Linear interpolation is possible for arbitrary angles between load direction and angle of the grain.

For the connection of two wooden members with different characteristic densities ρ_k for the determination of k_v may be determined by

$$\rho_k = \sqrt{\rho_{k,1} \cdot \rho_{k,2}}$$

with

$\rho_{k,1}$ characteristic density of wooden member 1 in kg/m^3

$\rho_{k,2}$ characteristic density of wooden member 2 in kg/m^3

Hilti screws

Characteristic data of the screws

Annex 5

of European Technical Assessment
ETA-22/0772 of 03.04.2023

A.5.3 Combined loading (perpendicular to and in direction of the screw axis)

Verification of Hilti screws under combined loading (perpendicular to and in direction of the screw axis) is performed by

$$\left(\frac{F_{ax,Ed}}{F_{ax,Rd}}\right)^2 + \left(\frac{F_{V,Ed}}{F_{V,Rd}}\right)^2 \leq 1$$

with

- $F_{ax,Ed}$ design value of the load in a connection in axial direction of the screws
- $F_{ax,Rd}$ design value of the load-bearing capacity of the screw connection in axial direction
- $F_{V,Ed}$ design value of the load in a connection in lateral direction of the screws
- $F_{V,Rd}$ design value of the load-bearing capacity of the screw connection in lateral direction

Hilti screws	Annex 5 of European Technical Assessment ETA-22/0772 of 03.04.2023
Characteristic data of the screws	

A7 Hilti screws for reinforcement of timber members for timber members loaded perpendicular to the grain and shear

A.7.1 Reinforcement of timber members loaded in compression perpendicular to the grain (reinforcement of supports)

The screws are driven into the timber member perpendicular to the contact surface under an angle between the screw axis and the grain direction of 45° to 90°. The screw heads must be flush with the timber surface.

Reinforcing screws for wood-based panels are not covered by this European Technical Assessment.

The design resistance of a reinforced contact area is:

$$R_{90,d} = \min \left\{ \begin{array}{l} k_{c,90} \cdot B_1 \cdot l_{ef,1} \cdot f_{c,90,d} + n \cdot \min \left(F_{ax,Rd}; \frac{N_{pl,k}}{\gamma_{M1}} \right) \\ B_2 \cdot l_{ef,2} \cdot f_{c,90,d} \end{array} \right. \text{ in N}$$

In addition to Clause A.5.1.6 the following parameters apply

- $k_{c,90}$ parameter considering the type of loading, the risk of splitting and the degree of the compression deformation according to EN 1995-1-1, 6.1.5
- B_1 bearing width in mm (minimum of steel plate and wooden member)
- l contact length in mm
- B_2 Width of the wooden member in the plane of the screw tip in mm
- $l_{ef,1}$ effective contact length according to EN 1995-1-1, 6.1.5, in mm
- $f_{c,d,90}$ design compressive strength perpendicular to the grain in N/mm²
- n number of reinforcing screws $n = n_0 \cdot n_{90}$
- n_0 number of reinforcing screws arranged in a row parallel to the grain
- n_{90} number of reinforcing screws arranged in a row perpendicular to the grain
- $l_{ef,2}$ effective contact length in the plane of the screw tips in mm
 - $l_{ef,2} = l_{ef} + (n_0 - 1) \cdot a_1 + \min(l_{ef}; a_{1,c})$ end supports
 - $l_{ef,2} = 2 \cdot l_{ef} + (n_0 - 1) \cdot a_1$ intermediate supports
- l_{ef} penetration length of the threaded part of the screw in the timber member in mm
- $a_{1,c}$ given spacing to end distance of the centre of gravity of the threaded part in the timber member in mm
- a_1 given spacing of Hilti screws in a plane parallel to the grain and screw axis
- γ_{M1} partial safety factor according to EN 1993-1-1

If the reinforcement screws are screwed into the wooden member from both sides and the following recommendations are observed, the second line in the Equation for calculation of the design resistance may be omitted.

Hilti screws	Annex 7 of European Technical Assessment ETA-22/0772 of 03.04.2023
Reinforcement with Hilti screws	

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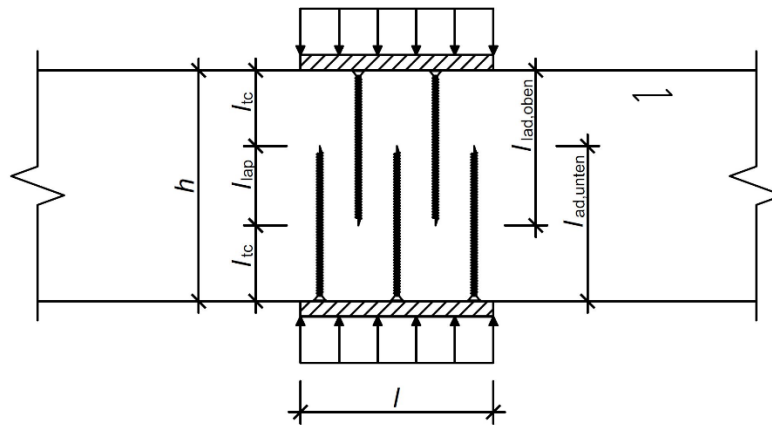


Figure A7.2: Reinforcement of timber members loaded in compression perpendicular to the grain for load transfer

A.7.2 Reinforcement of timber members loaded in tension perpendicular to the grain

Fully threaded screws may be used as tensile reinforcement perpendicular to the grain of the timber members. The screws are driven into the timber member under an angle between the screw axis and the grain direction of 90°. A minimum of two screws shall be used for tensile reinforcement perpendicular to the grain. Only one screw may be used when the minimum penetration depth of the screws below and above the potential crack is 20 · d where d is the outer thread diameter of the screw.

A.7.2.1 Tension reinforcement for transverse connections and notches

Tension reinforcement of transverse connections and notches in wooden members may be designed as follows:

$$1.3 \cdot V_d \cdot \left[3 \cdot \left(1 - \frac{h_{ef}}{h} \right)^2 - 2 \cdot \left(1 - \frac{h_{ef}}{h} \right)^3 \right] \leq F_{ax,Rd} \quad \text{for notches}$$

$$F_{90,Ed} \cdot \left[1 - 3 \cdot \left(\frac{a}{h} \right)^2 + 2 \cdot \left(\frac{a}{h} \right)^3 \right] \leq F_{ax,Rd} \quad \text{for transverse connections}$$

where

$$F_{ax,Rd} = n_{90} \cdot \min \left\{ \begin{array}{l} f_{ax,k} \cdot d \cdot l_{ef} \cdot \frac{k_{mod}}{\gamma_M} \\ \frac{f_{tens,k}}{\gamma_{M2}} \end{array} \right. \quad \text{for reinforcement acc. to Figure A7.3 and A7.4}$$

- V_d design value of the lateral force in N
- $F_{90,Ed}$ design value of the force acting in the connection perpendicular to the grain of the timber members in N
- h_{ef} effective height/thickness of the timber member above the notch in mm
- h height/thickness of the timber member in mm

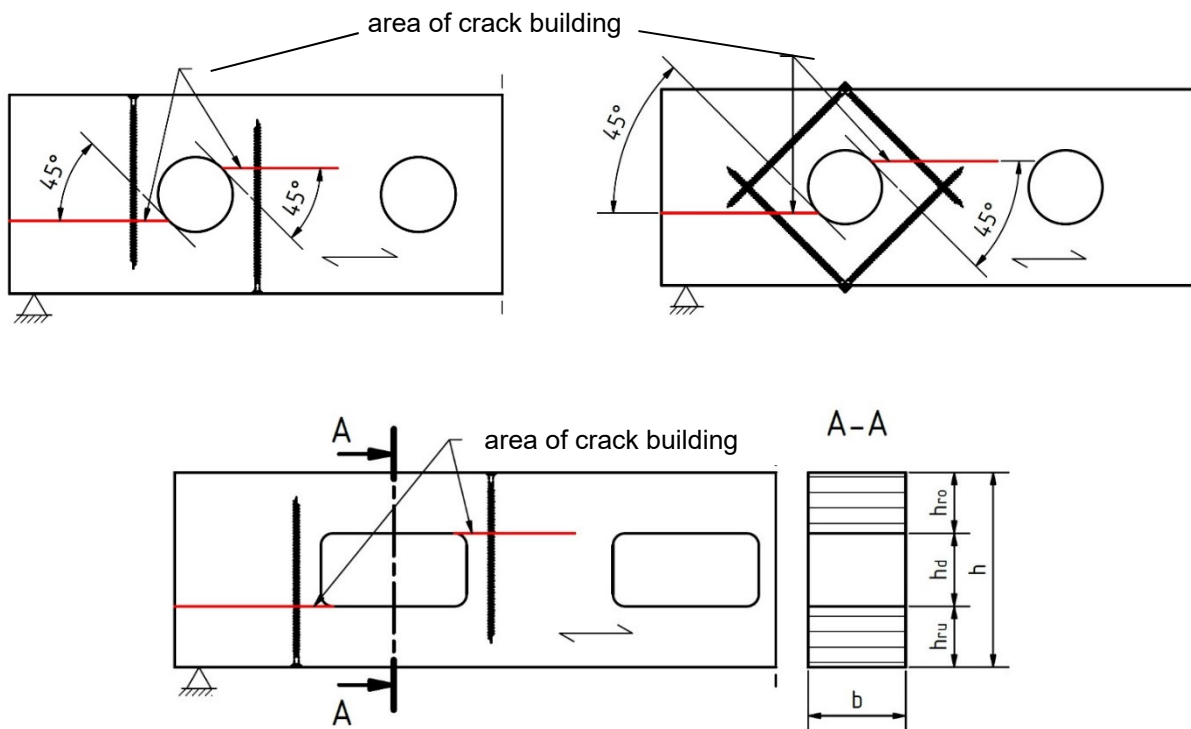
Hilti screws	Annex 7 of European Technical Assessment ETA-22/0772 of 03.04.2023
Reinforcement with Hilti screws	

electronic copy

$$F_{ax,Rd} = n_{90} \cdot \min \left\{ \begin{array}{l} f_{ax,k} \cdot d \cdot l_{ef} \cdot \frac{k_{mod}}{\gamma_M} \\ \frac{f_{tens,k}}{\gamma_{M2}} \end{array} \right. \text{ for reinforcement acc. to Figure A7.5}$$

with

- $F_{t,V,d}$ design value of tension force perpendicular to the grain due to lateral force V_d in N
- $F_{t,M,d}$ design value of tension force perpendicular to the grain due to bending moment M_d in N
- h_d height of the opening for rectangular openings or 70 % of opening diameter for circular openings in mm
- h_r $\min(h_{ro}; h_{ru})$ for rectangular openings or $\min(h_{ro} + 0.15 h_d; h_{ru} + 0.15 h_d)$ for circular openings in mm
- l_{ef} smaller value of the penetration depth below or above the plane of the potential crack in mm
- k_{mod} modification factor for duration of load and moisture content according to EN 1995 1-1
- γ_M partial safety factor for connections according to EN 1995-1-1, Table 2.3
- γ_{M2} partial safety factor according to EN 1993-1-1
- n_{90} number of reinforcing screws arranged in a row perpendicular to the grain



Hilti screws

Reinforcement with Hilti screws

Annex 7

of European Technical Assessment
ETA-22/0772 of 03.04.2023

A.7.2.4 Shear reinforcement

Fully threaded screws may be used as shear reinforcement of solid timber, glued laminated timber and glued solid timber of softwood. The provisions are valid for straight beams with constant rectangular cross-section. The screws are driven into the timber member under an angle between the screw axis and the grain direction of 45°.

A minimum of four screws shall be used for shear reinforcement in a line parallel to the grain whereas the spacing between the screws shall not exceed the depth *h* of the timber member. If the screws are arranged in one line parallel to the grain, it shall be done centrally in relation to the beam width.

The effect of the reinforcement is limited to the shaded part of the timber member. Outside this area sufficient shear strength of the cross section must be verified.

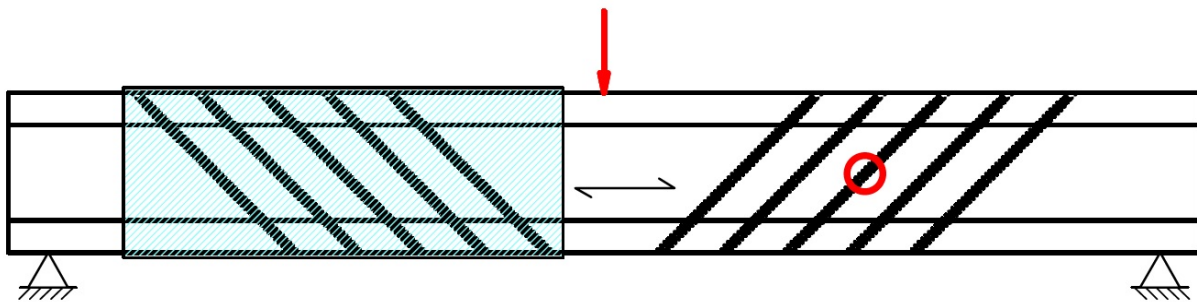


Figure A7.7: Shear reinforcement with Hilti screws

Shear reinforcement may be designed according to

$$\tau_d \leq \frac{f_{v,d} \cdot \kappa_\tau}{\eta_H}$$

where

τ_d design value of shear stress in N/mm²

$f_{v,d}$ design value of shear strength in N/mm²

$$\kappa_\tau = 1 - 0.46 \cdot \sigma_{90,d} - 0.052 \cdot \sigma_{90,d}^2$$

$\sigma_{90,d}$ design value of stress perpendicular to the grain in N/mm²

$$\sigma_{90,d} = \frac{F_{ax,d}}{\sqrt{2} \cdot b \cdot a_1}$$

b width of the timber member in mm

*a*₁ spacing of screws parallel to the grain in mm

$$F_{ax,d} = \frac{\sqrt{2} \cdot (1 - \eta_H) \cdot V_d \cdot a_1}{h}$$

*V*_d design shear force in N

h height of the timber member in mm

Hilti screws	Annex 7 of European Technical Assessment ETA-22/0772 of 03.04.2023
Reinforcement with Hilti screws	

electronic copy

A.8.1 Fastening of thermal insulation material (on top of rafters and facades)

Hilti screws with an outer thread diameter of at least 6 mm and lengths between 120 mm and 600 mm may be used for fixing of thermal insulation material on rafters or on wood-based members in vertical facades. Screws with partial thread and cylinder head are excluded from fixing wood-based panels on rafters with thermal insulation material as interlayer.

The angle between grain direction and screw axis shall be $30^\circ \leq \alpha \leq 90^\circ$.

The thickness of the **thermal insulation material** is max. 400 mm. The thermal insulation material shall be applicable as insulation on top of rafters according to national provisions that apply at the installation site.

The **battens** are made from solid timber strength class C24 according to EN 338 and EN 14081-1. The minimum thickness and width of the battens is:

Table A8.1 Minimum thickness and width of the battens

Screw diameter d in mm	b_{min}	t_{min}
	mm	mm
≤ 8	50	30
10	60	40
12	80	50

Instead of battens the following **wood-based panels** may be used to cover the thermal insulation material if they are suitable for that use:

- Plywood according to EN 636 and EN 13986,
- Oriented Strand Board, OSB according to EN 300 and EN 13986,
- Particleboard according to EN 312 and EN 13986
- Fibreboards according to EN 622-2, EN 622-3 and EN 13986.

The minimum thickness of the wood-based panels shall be 22 mm.

The word batten in the following includes the meaning of the above mentioned wood-based panels.

The **substructure** is made from solid timber strength class C24 according to EN 338 and EN 14081-1, cross laminated timber according to European Technical Assessments or laminated veneer lumber according to EN 14374. The minimum width is $b_{min} = 60$ mm, for screws with an outer thread diameter of 12 mm the minimum width $b_{min} = 80$ mm.

The spacing between screws e_s shall be not more than 1.75 m.

Friction forces shall not be considered for the design of the characteristic axial capacity of the screws.

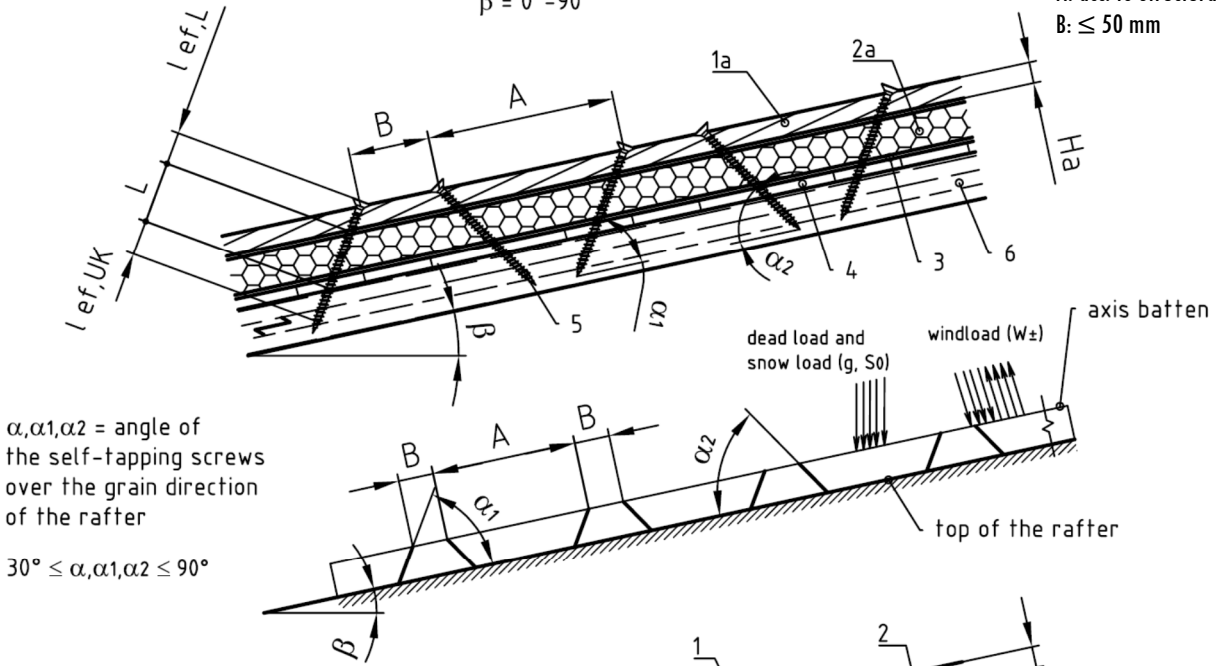
Hilti screws	Annex 8 of European Technical Assessment ETA-22/0772 of 03.04.2023
Fastening of thermal insulation material	

electronic copy electronic copy electronic copy electronic copy electronic copy electronic copy electronic copy electronic copy electronic copy electronic copy

Version 1

roof, facade
 $\beta = 0^\circ - 90^\circ$

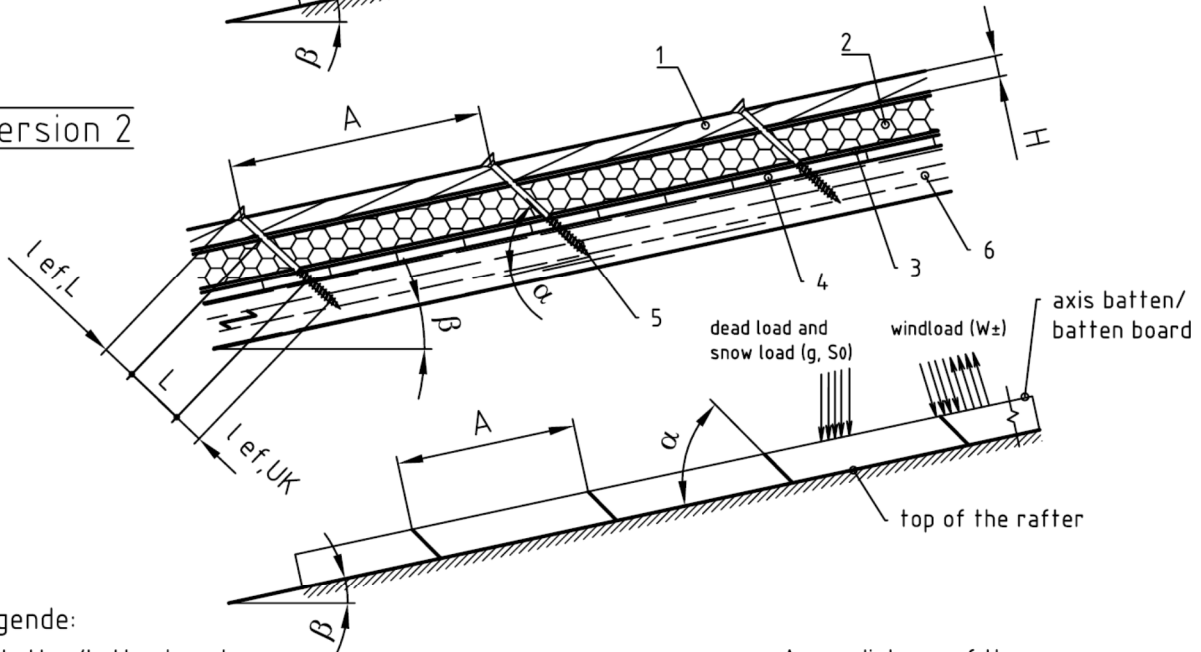
A: acc. to structural analysis
B: ≤ 50 mm



$\alpha, \alpha_1, \alpha_2 =$ angle of the self-tapping screws over the grain direction of the rafter

$30^\circ \leq \alpha, \alpha_1, \alpha_2 \leq 90^\circ$

Version 2



Legende:

- 1 batten/batten board
- 1a batten
- 2 heat insulation (till 300mm), pressure resistant (min. 0,05) N/mm²
- 2a heat insulation (till 400mm), not pressure resistant
- 3 vapour barrier
- 4 roof boards
- 5 self-tapping screws
- 6 rafter

- A distance of the screws
- H thickness batten/batten board
- Ha thickness batten
- l ef,L penetration length in the batten/batten board
- l ef,UK penetration length in the rafter

Hilti screws

Annex 8

Fastening of thermal insulation material

of European Technical Assessment
ETA-22/0772 of 03.04.2023

Table A8.2 Buckling capacity of the screws

Free screw length <i>l</i> between batten and rafter (mm)	$\kappa_c \cdot N_{pl,k}$ (kN) for Hilti screws		
	Outer thread diameter <i>d</i>		
	8	10	12
	Inner thread diameter <i>d_i</i>		
	5.2	6.2	6.9
≤35	11.681	19.024	25.125
60	7.576	13.516	18.834
80	5.416	10.070	14.470
100	4.008	7.621	11.154
120	3.068	5.912	8.747
140	2.418	4.699	7.000
160	1.952	3.815	5.710
180	1.608	3.156	4.739
200	1.347	2.652	3.992
220	1.144	2.259	3.407
240	0.984	1.947	2.941
260	0.855	1.695	2.563
280	0.750	1.489	2.254
300	0.663	1.318	1.997
320	0.591	1.175	1.781
340	0.529	1.054	1.599
360	0.477	0.950	1.443
380	0.432	0.862	1.309
400	0.393	0.785	1.193

A.8.3 Parallel inclined screws

The screws are predominantly loaded in tension whereas corresponding thermal insulation material is loaded in compression. The minimum compression stress of the thermal insulation material at 10 % deformation, measured according to EN 826, shall be $\sigma_{(10\%)} \geq 0.05 \text{ N/mm}^2$. Hereby systems with battens or wood-based panels may be used.

Design

For design of thermal insulation systems in terms of number and spacing of the screws the following characteristic withdrawal parameter may be taken into account:

$$R_{ax,k} = \min \left\{ \begin{array}{l} f_{ax,k,\alpha} \cdot d \cdot l_{ef,UK} \cdot k_1 \cdot k_2 \\ \max \left\{ \begin{array}{l} f_{head,k} \cdot d_k^2 \\ f_{ax,k,\alpha} \cdot l_{ef,L} \cdot d \end{array} \right. \end{array} \right. \quad \text{in N}$$

Hilti screws	Annex 8
Fastening of thermal insulation material	of European Technical Assessment ETA-22/0772 of 03.04.2023

where:

$f_{ax,k,\alpha}$ = characteristic value of the axial withdrawal parameter of the threaded part of the screw in the batten, $f_{ax,k,\alpha}$ does not apply for wood-based panels

$f_{head,k}$ = characteristic head pull-through parameter according to Tables A5.6 and A5.7

$$k_1 = \min \left\{ \begin{array}{l} 1 \\ \frac{220}{d_{D\bar{a}}} \end{array} \right.$$

$$k_2 = \min \left\{ \begin{array}{l} 1 \\ \frac{\sigma_{10\%}}{0.12} \end{array} \right.$$

$d_{D\bar{a}}$ = thickness of thermal insulation material in mm

$\sigma_{10\%}$ = compressive stress of thermal insulation material at 10 % strain in N/mm²

Hilti screws

Annex 8

Fastening of thermal insulation material

of European Technical Assessment
ETA-22/0772 of 03.04.2023

European Assessment Document EAD 130118-01-0603 “Screws and threaded rods for use in timber constructions”

EN 300 (07.2006), Oriented Strand Boards (OSB) – Definitions, classification and specifications

EN 312 (09.2010), Particleboards – Specifications

EN 338 (04.2016), Structural timber – Strength classes

EN 622-2 (04.2004) +AC (12.2005), Fibreboards – Specifications – Part 2: Requirements for hardboards

EN 622-3 (04.2004), Fibreboards – Specifications – Part 3: Requirements for medium boards

EN 634-1 (03.1995), Cement-bonded particleboards – Specifications – Part 1: General requirements

EN 636:2012+A1 (03.2015), Plywood – Specifications

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